

# Investigating the Influence of RAP Source Consistency on the Performance and Service Life of High-RAP Content Mixtures





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#### **Outline**

- **Background**
- **≻**Objectives
- > Experimental Plan
- ➤ Pilot Projects
- > Results
- ➤ Pavement Performance Prediction



# **Background**

- MassDOT's current specifications only allow up to 15% RAP in its surface asphalt mixture layer.
- ➤ Based on a 2020 MassDOT study entitled *Influence of RAP Source and Virgin Binder Source on RAP Specifications and Balanced Mix Design* the following was concluded:
  - The RAP content could be increased over the 15% maximum based on the properties of the RAP, which depends on its source.
  - For the same RAP content, RAP source has a significant effect on the cracking resistance of asphalt mixtures.



# **Background**

- ➤ MassDOT approved demonstration projects using high RAP surface asphalt mixtures with RAP contents between 25% and 30%.
- ➤ Subsection 454 "Superpave High RAP Surface Course."



# Subsection 454 "Superpave High RAP Surface Course"

**MassDOT Performance Testing Requirements for SSC-HR** 

Performance Characteristic	Test Method	Requirement
HWTD Maximum Rut Depth	AASHTO T 324 @ 45°C (1)	< 0.5 inch
Beam Fatigue Test Failure	AASHTO T 321 @ 25°C (2)	For Information Only
Dynamic Modulus	AASHTO T 342	For Information Only
Semi-Circular Bend, Flexibility Index (FI)	AASHTO TP 124 @ 25°C	For Information Only
IDEAL-CT	ASTM E1169	For Information Only
TSRST	EN 12697-46	For Information Only

<sup>(1)</sup> Maximum rut depth after 20,000 passes.

The final blended binder grade (RAP and Virgin) of the SSC-HR after extraction and recovery shall be a PG64E-28



<sup>(2)</sup> Test performed at a strain level of 500 microstrains.

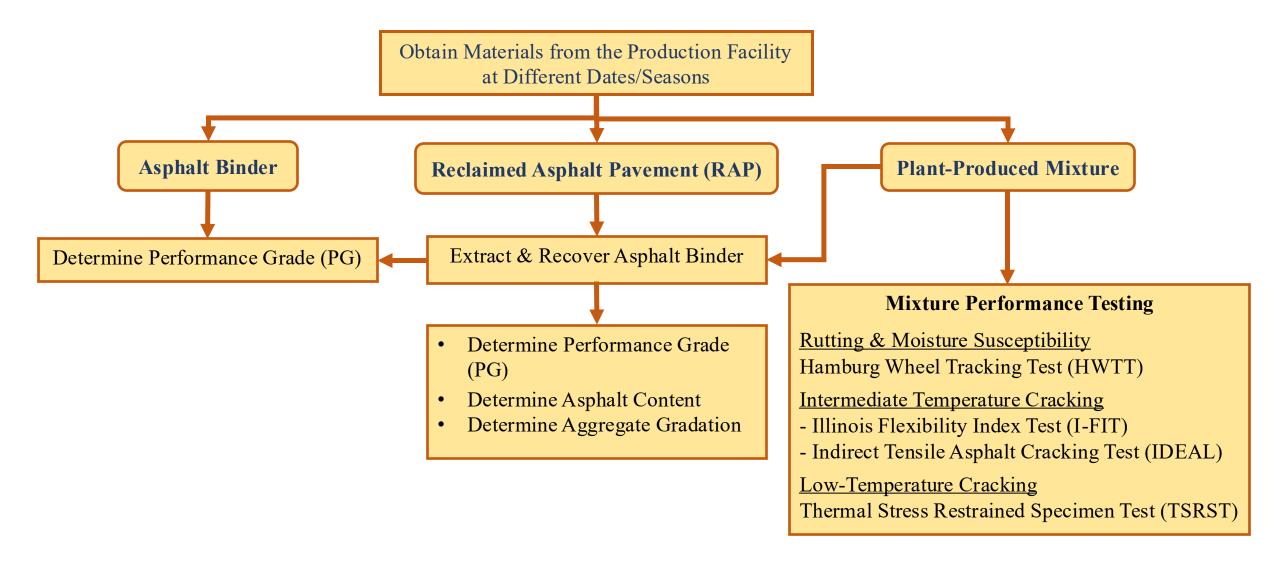
# **Objectives**

#### **Across Different Seasons:**

- 1. Document variability in virgin binder, RAP, and mixtures.
- 2. Characterize the impact of virgin binder and RAP properties on mixture performance, and
- 3. Evaluate the influence of material variations during production on the balanced performance of mixtures.



# **Experimental Plan**





# **Pilot Projects**

Contactor #	#1	#2	#3	7	<b>#4</b>
NMAS (mm)	9.5	12.5	12.5	12.5	9.5
RAP Content (%)	25	28	27		30
N <sub>des</sub> (Gyrations)	100	100	100	,	75
PG Binder	PG64E-34	PG64E-34	PG64E-34	PG6	4E-34
Additives	WMA	WMA	WMA	W	MA
Sample Date	<ul> <li>LTMF</li> <li>10/21</li> <li>11/21</li> <li>06/22</li> <li>05/23</li> </ul>	<ul> <li>LTMF</li> <li>10/22</li> <li>06/23</li> <li>08/23</li> </ul>	<ul><li>LTMF</li><li>06/24</li></ul>	<ul><li>LTMF</li><li>06/24</li></ul>	• LTMF



# Asphalt Binder Test Results - Virgin Binders

Contractor #1			
Sample Date	Binder PG	Average ΔT <sub>C</sub>	
LTMF	PG64(64E) -34	+0.8	
10/21	PG70(64E) -28	-0.3	
11/21	PG64(64E) -34	+0.7	
06/22	PG70(64E) -34	+0.3	
05/23	PG64(64E) -34	+0.5	

Contractor #2		
Sample Date	Binder PG	Average ΔT <sub>C</sub>
LTMF	PG70(64E) -34	+0.1
10/22	PG70(64E) -34	+0.3
06/23*	PG64(64V) -28	+0.5
08/23	PG64(64V) -34	+0.7

<sup>\*</sup> Virgin binder 64E-28 with recycling agent

The final blended binder grade (RAP and virgin) in the mixture should be a PG64E-28.

Contractor #3		
Sample Date	Binder PG	Average ΔT <sub>C</sub>
LTMF	PG64(64E) -34	-0.6
06/24	PG70(64E) -34	-0.4

Contractor #4		
Sample Date	Binder PG	Average ΔT <sub>C</sub>
LTMF	PG64(64E) -34	+0.4
06/24	NT	

NT =Not Tested



# Asphalt Binder Test Results - RAP Binders

Contractor #1			
Sample Date	Binder PG	Average $\Delta T_{\rm C}$	
LTMF	PG88-16	-2.9	
10/21	PG88-16	-2.8	
11/21	PG82-22	-2.3	
06/22	PG82-22	-1.1	
05/23	PG88-16	-3.4	

Contractor #2			
Sample Date	Binder PG	Average ΔT <sub>C</sub>	
LTMF	PG88-16	-1.6	
10/22	PG88-16	-1.9	
06/23	PG88-16	0.0	
08/23	PG88-16	-0.7	

- The RAP binder high- and low-temperature grades changed by one grade from LTMF to production in some instances.
- This highlights the need to verify the RAP stockpile properties during production.

Contractor #3		
Sample Date	Binder PG	Average ΔT <sub>C</sub>
LTMF	88-16	-2.8
06/24	88-16	-1.2

Contractor #4		
Sample Date	Binder PG	Average $\Delta T_{\rm C}$
LTMF	88-16	-1.3
06/24	NT	

NT=Not Tested



# Asphalt Binder Test Results - Mixture Binders

Contractor #1			
Sample Date	Binder PG	Average ΔT <sub>C</sub>	
LTMF	PG76(64E) -28	-1.1	
10/21	PG70(64V) -28	-1.0	
11/21	PG70(64E) -28	-2.2	
06/22	PG70(64V) -22	-2.4	
05/23	PG70(64V) -28	1.2	

Contractor #2			
Sample Date	Binder PG	Average ΔT <sub>C</sub>	
LTMF	PG88(64E) -22	-3.5	
10/22	PG82(64E) -22	-1.0	
06/23	PG82(64E) -22	-1.3	
08/23	PG76(64V) -28	+0.2	

Contractor #3						
Sample Date	Binder PG	Average $\Delta T_{C}$				
LTMF	PG70(64E) -28	0.0				
06/24	PG70(64E) -28	+0.6				

Contractor #4								
Sample Date	Binder PG	Average ΔT <sub>C</sub>						
LTMF (12.5 mm)	PG76(64E) -22	+1.4						
LTMF (9.5 mm)	PG76(64E) -28	+0.2						



# Asphalt Binder Test Results - Mixture Binders

➤ Not all extracted and recovered mixture binders met the MassDOT specified criteria of a PG64E-28.



Contractor #1 (25% RAP)									
	LTMF	10/21	11/21	06/22	05/23	Standard Deviation	Suggested NCHRP 752 Standard Deviation Limits		
19 mm (3/4")	100	100	100	100	100	0	< 5.0		
12.5 mm (3/4")	98.1	97.3	98.5	97.6	98.8	0.63	< 5.0		
9.5 mm (3/4")	92.4	90.6	90.8	88.2	93.4	1.9	< 5.0		
4.75 mm (No. 4)	69.7	71.9	63.5	61.2	73.7	5.4 F	< 5.0		
2.36 mm (No. 8)	51.9	57.0	46.1	44.2	57.4	6.05 F	< 5.0		
1.18 mm (No. 16)	39.0	44.0	33.8	32.7	44.1	5.41 F	< 5.0		
0.6 mm (No. 30)	28.4	32.1	24.5	24.1	32.4	3.99	< 5.0		
0.3 mm (No. 50)	18.8	21.7	16.7	17.1	21.8	2.46	< 5.0		
0.15 mm (No. 100)	11.7	13.5	10.3	11.0	13.5	1.44	< 5.0		
0.075 mm (No. 200)	7.5	8.7	6.5	7.1	8.5	0.93	< 1.5		
Binder Content, %	4.88	5.37	5.15	5.00	5.46	0.24	< 0.5		

Note: F= Standard deviation of measurements outside suggested NCHRP 752 limits



Contractor #2 (28% RAP)									
	LTMF	10/21	11/21	06/22	Standard Deviation	Suggested NCHRP 752 Standard Deviation Limits			
19 mm (3/4")	100.0	100.0	100.0	100.0	0	< 5.0			
12.5 mm (3/4")	98.5	98.5	99.0	99.0	0.28	< 5.0			
9.5 mm (3/4")	93.4	94.4	95.4	93.1	1.06	< 5.0			
4.75 mm (No. 4)	65.7	71.1	70.8	73.3	3.22	< 5.0			
2.36 mm (No. 8)	45.4	53.4	50.1	56.0	4.56	< 5.0			
1.18 mm (No. 16)	33.3	41.4	37.7	40.9	3.74	< 5.0			
0.6 mm (No. 30)	24.8	31.3	28.9	29.5	2.76	< 5.0			
0.3 mm (No. 50)	17.8	22.2	21.3	20.9	1.95	< 5.0			
0.15 mm (No. 100)	11.6	15.0	14.9	14.1	1.56	< 5.0			
0.075 mm (No. 200)	7.7	10.7	10.7	9.5	1.41	< 1.5			
<b>Binder Content, %</b>	6.08	6.16	6.06	6.04	0.05	< 0.5			



Contractor #3 (27% RAP)									
	LTMF 06/24 Standard Deviation								
19 mm (3/4")	100.0	100.0	n/a	< 5.0					
12.5 mm (3/4")	99.2	99.9	n/a	< 5.0					
9.5 mm (3/4")	92.2	92.3	n/a	< 5.0					
4.75 mm (No. 4)	61.5	67.4	n/a	< 5.0					
2.36 mm (No. 8)	42.1	50.3	n/a	< 5.0					
1.18 mm (No. 16)	32.3	38.5	n/a	< 5.0					
0.6 mm (No. 30)	25.2	29.1	n/a	< 5.0					
0.3 mm (No. 50)	18.5	20.7	n/a	< 5.0					
0.15 mm (No. 100)	12.1	13.4	n/a	< 5.0					
0.075 mm (No. 200)	7.9	8.9	n/a	< 1.5					
<b>Binder Content, %</b>	5.39	5.77	n/a	< 0.5					

Note: n/a = not applicable.



Contractor #4 (30% RAP)								
	LTMF	Standard Deviation	Suggested NCHRP 752 Standard Deviation Limits					
19 mm (3/4")	100.0	n/a	< 5.0					
12.5 mm (3/4")	99.8	n/a	< 5.0					
9.5 mm (3/4")	95.6	n/a	< 5.0					
4.75 mm (No. 4)	69.6	n/a	< 5.0					
2.36 mm (No. 8)	50.9	n/a	< 5.0					
1.18 mm (No. 16)	37.1	n/a	< 5.0					
0.6 mm (No. 30)	25.4	n/a	< 5.0					
0.3 mm (No. 50)	16.9	n/a	< 5.0					
0.15 mm (No. 100)	11.4	n/a	< 5.0					
0.075 mm (No. 200)	8.1	n/a	< 1.5					
<b>Binder Content, %</b>	6.44	n/a	< 0.5					

Note: n/a = not applicable.



### Mixture Property Test Results

Contractor #1 (25% RAP)									
	LTMF	10/21	11/21	06/22	05/23	9.5 mm Superpave	Lower Engineering	Upper Engineering	
		10,21		0 0. ==	00,20	<b>Specification</b>	Limit (LEL)	Limit (UEL)	
19 mm (3/4")	100	100	100	100	100	100	100	100	
12.5 mm (3/4")	100	98.2 F	98.9 F	99.5 F	99.9 F	100 min	100	100	
9.5 mm (3/4")	94.0	92.6	93.3	95.2	96.4	90-100	90	100	
4.75 mm (No. 4)	62.0	59.8	59.1	68.2 F	66	90 max	56	68	
2.36 mm (No. 8)	40.0	39.7	39.1	47.3 F	43.1	32-67	35	45	
1.18 mm (No. 16)	29.0	27.1	26.8	31.6	29	-	26	32	
0.6 mm (No. 30)	20.0	18.4	18.5	21.2	19.5	-	17	23	
0.3 mm (No. 50)	13.0	12.2	12.6	12.8	12.6	1	10	16	
0.15 mm (No. 100)	8.0	7.6	8	7.6	7.4	ı	6	10	
0.075 mm (No. 200)	4.0	4.6	5.3	4.3	4.6	2-10	2.5	5.5	
<b>Binder Content, %</b>	5.60	5.74	5.91	6.1 F	5.60	-	5.2	6.0	
$G_{mm}$	2.471	2.474	2.473	2.501*	2.471	-	-	-	

#### F= Outside MassDOT acceptance limit



<sup>\*</sup> Significantly different G<sub>mm</sub> compared to the LTMF.

## Mixture Property Test Results

Contractor #2 (28% RAP)									
	LTMF	10/22	06/23	08/23	12.5 mm Superpave Specification	Lower Engineering Limit (LEL)	Upper Engineering Limit (UEL)		
19 mm (3/4")	100	100	100	100	100	100	100		
12.5 mm (3/4")	95.0	93.1	97.4	96.8	90-100	89	100		
9.5 mm (3/4")	81.0	78.6	85.5	81.0	90 max	75	87		
4.75 mm (No. 4)	51.0	47.7	53.8	49.9	-	45	57		
2.36 mm (No. 8)	34.0	32.6	33.8	32.7	28-58	29	39		
1.18 mm (No. 16)	26.0	24.2	24.2	23.8	-	23	29		
0.6 mm (No. 30)	19.0	18.6	18.5	18.0	-	17	23		
0.3 mm (No. 50)	14.0	14.2	14.3	13.7	-	11	17		
0.15 mm (No. 100)	8.0	9.2	9.7	9.1	-	6	10		
0.075 mm (No. 200)	4.3	5.6	5.9 F	5.5	2-10	2.8	5.8		
<b>Binder Content, %</b>	5.20	5.26	5.60	5.20	-	4.8	5.6		
$G_{mm}$	2.473	2.478	2.479	2.479	-	-	-		

F= Outside MassDOT acceptance limit

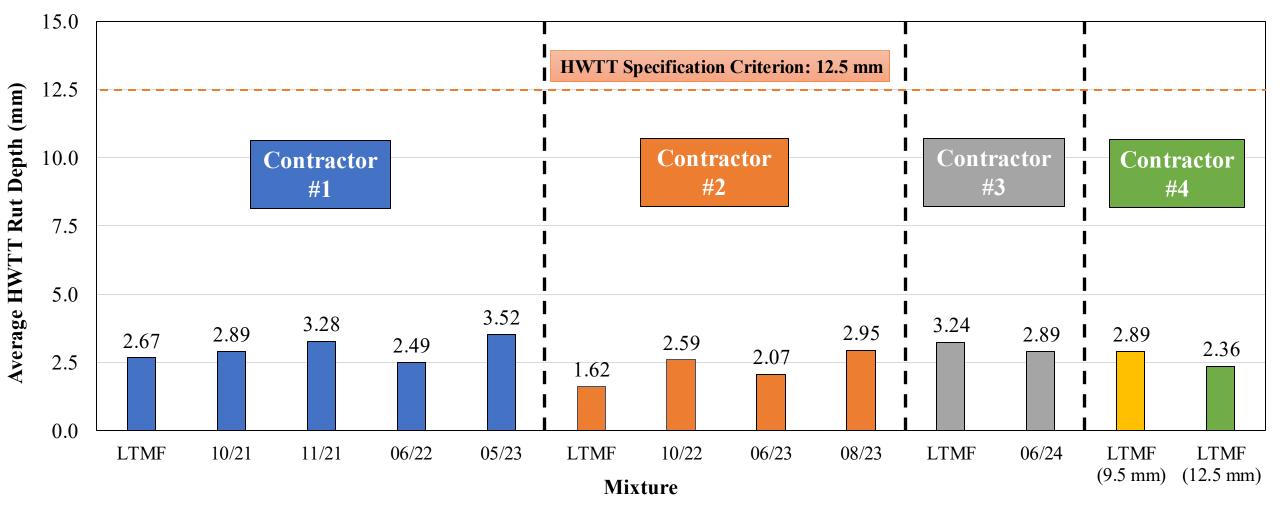


#### **Mixture Performance Evaluation**

	Rutting & Moisture Susceptibility	Intermediate Temp Tes		Reflective Cracking	Low Temperature Cracking
	HWTT	I-FIT	IDEAL-CT	Overlay Test	TSRST
Test					
Specification	AASHTO T 324	AASHTO T 393	ASTM D 8225	Tex-248-F	AASHTO TP 10-93
Test Temperature	45°C	25°C	25°C	25°C	n/a



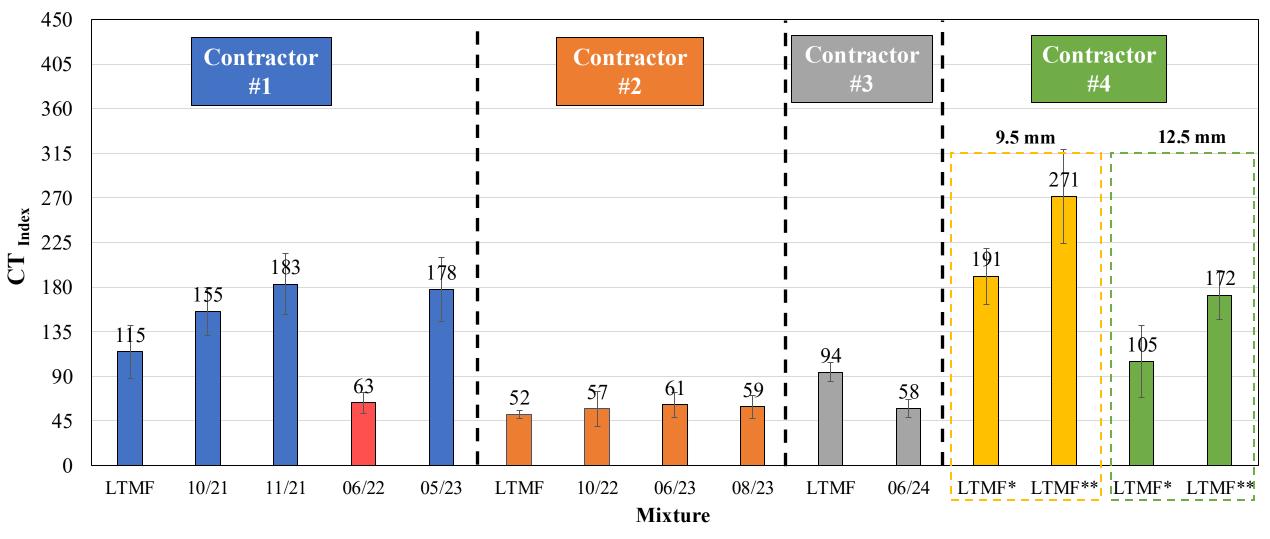
# Mixture Rutting and Moisture Susceptibility Results



- All mixtures met the MassDOT specification criteria.
- Rutting and moisture damage were not issues for these mixtures.



# Mixture Intermediate Temperature Cracking Results

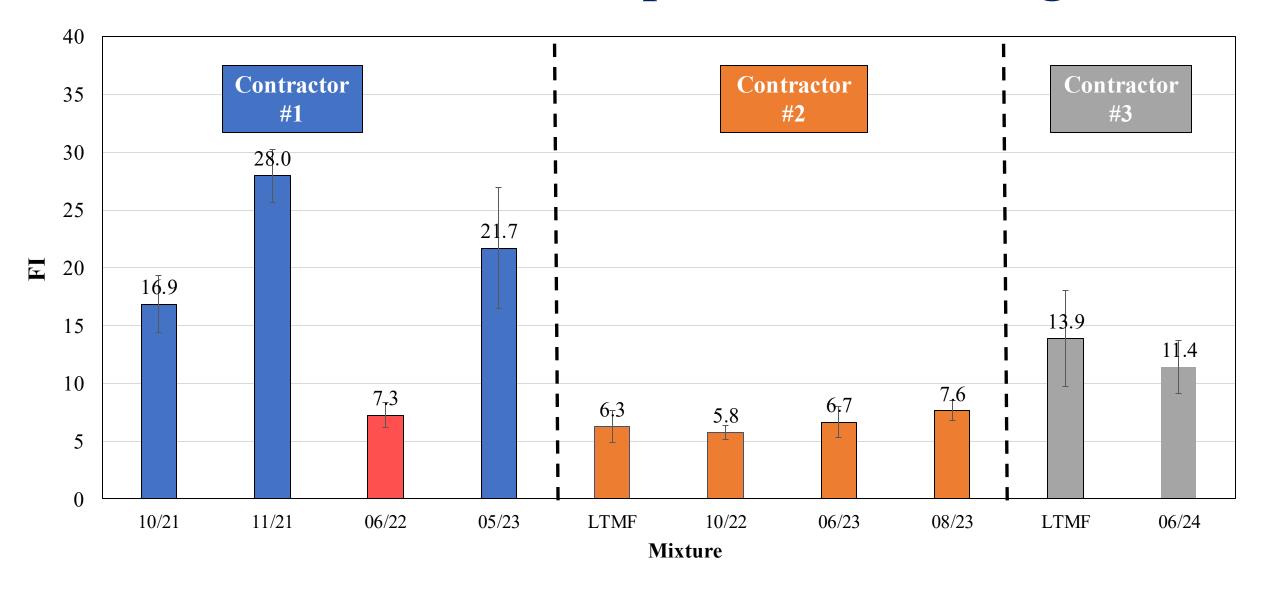


<sup>\*</sup>Compacted specimens received from Contractor #4.



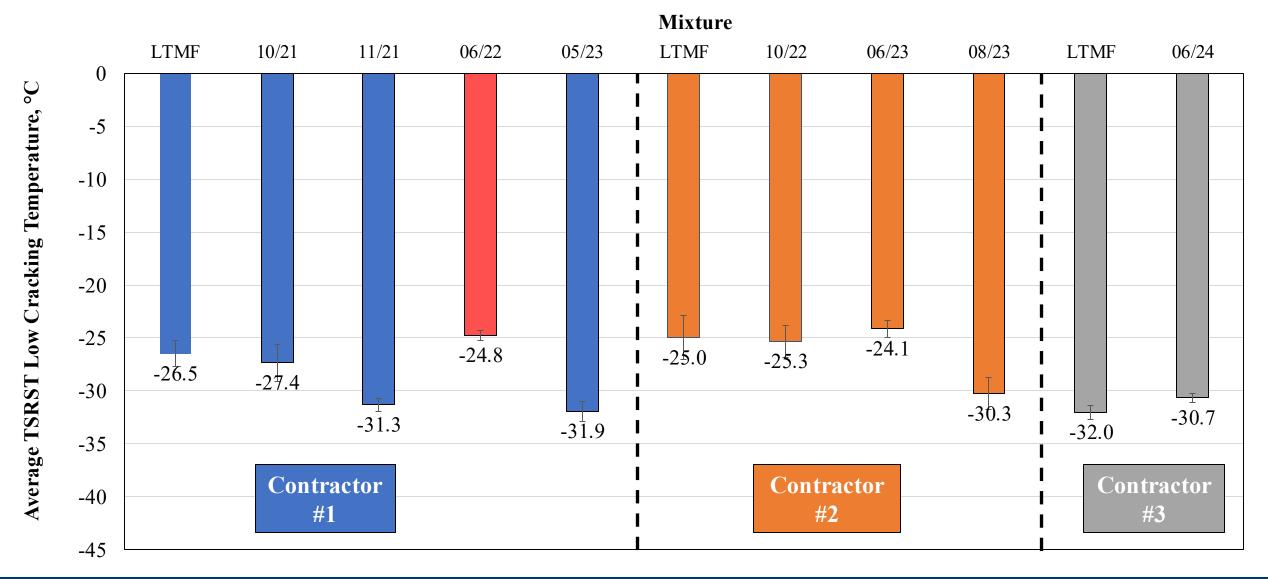
<sup>\*\*</sup>Loose mixture received from Contractor #4 and reheated and compacted at the HSRC.

## Mixture Intermediate Temperature Cracking Results





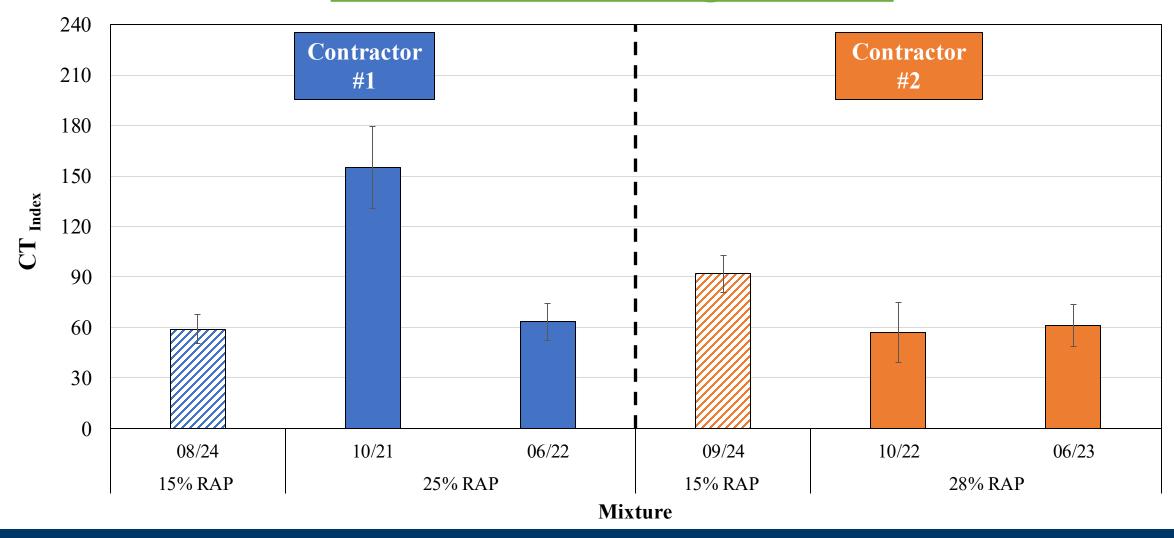
# Mixture Low-Temperature Cracking Results





### Mixture Intermediate Temperature Cracking Results

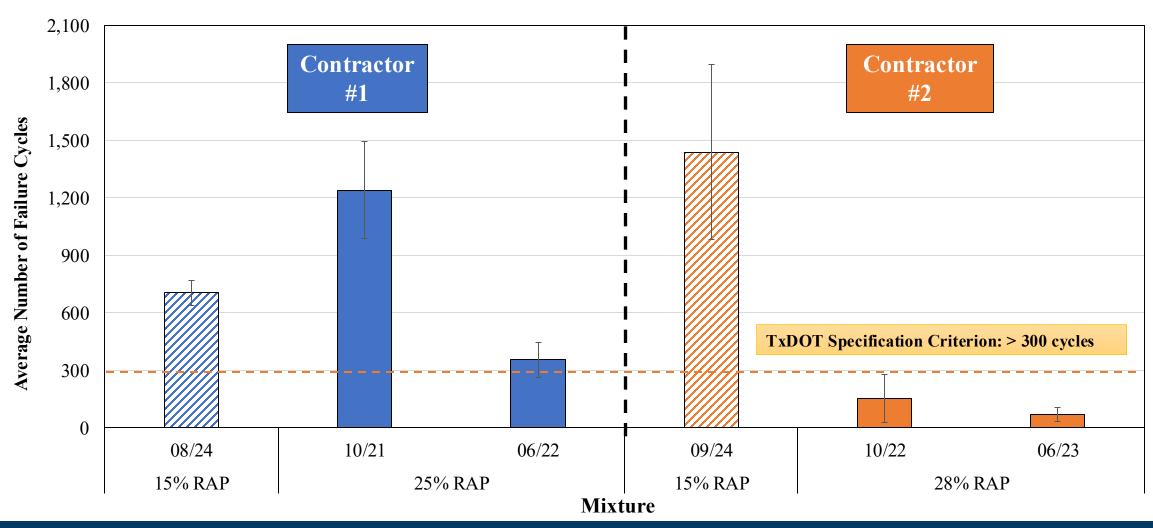
(15% RAP vs. High RAP)





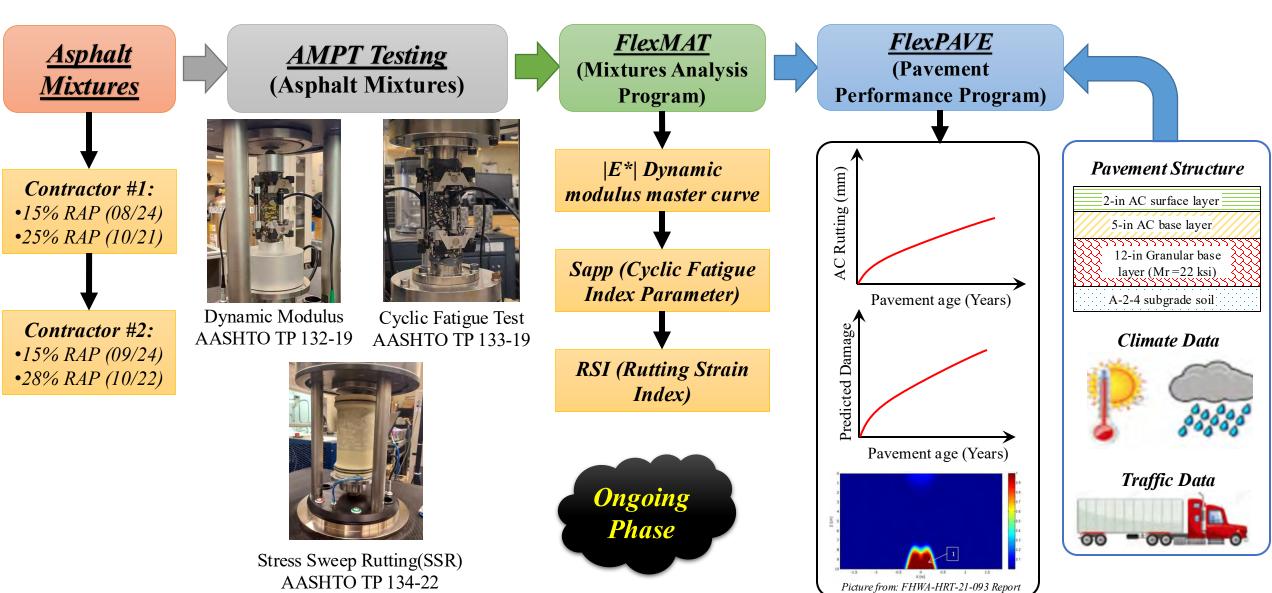
## Mixture Reflective Cracking Results

(15% RAP vs. High RAP)

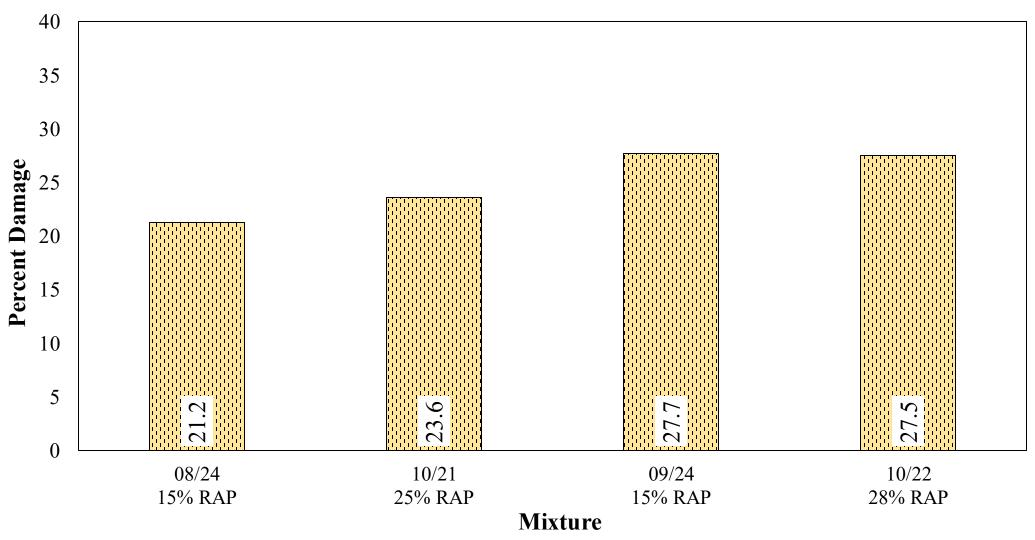




#### Pavement Performance Prediction Using FlexPAVE<sup>TM</sup> Software



#### Pavement Performance Prediction Using FlexPAVE<sup>TM</sup> Software



Predicted percent total damage after 20 years for the simulated pavement section



#### **Discussion of Results**

- > RAP stockpile consistency proved critical to mixture performance.
- ➤ All SSC-HR mixtures satisfied MassDOT's rutting and moisture susceptibility.
- ➤ Low- and intermediate-temperature cracking resistances were generally acceptable when compared to corresponding control LTMF design mixture.
- SSC-HR mixtures can achieve balanced performance when RAP variability, binder grade, and mixture properties are adequately controlled.
- ➤ FlexMAT<sup>TM</sup> and FlexPAVE<sup>TM</sup> analyses indicated that SSC-HR mixtures can provide long-term rutting and fatigue performance comparable to conventional 15% RAP mixtures.





